



Land at Pontymister, Risca

Hydraulic Modelling Report

January 2025



Scheme Information	
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Report Title:	Hydraulic Modelling Report
Client:	Lidl Great Britain Limited
Instruction:	The instruction to undertake this modelling was received from Rhydian Griffiths of Lidl Great Britain Limited.
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Supporting Documents:	This Hydraulic Modelling Report should be read in conjunction with the associated Flood Consequences Assessment and Drainage Strategy written by Waterco in November 2024 (15679-FCA and Drainage Strategy-02).

Approval Record	
Author:	Martha Hughes BSc (Hons) MSc
Checker:	Awel Agotz BSc (Hons) MCIWEM
Approver:	Laura Smith BSc (Hons) MCIWEM C.WEM

Document History		
Revision	Date	Comment
1	29/11/2024	First issue
2	24/01/2025	Second issue – Updated with revised site layout

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This report will remain valid for a period of twelve months (from the date of last issue) after which the source data should be reviewed in order to reassess the findings and conclusions on the basis of latest available information.

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Glossary and Modelling Short Codes

Table 1: Glossary of Technical Terms

Term / Acronym	Definition
1D (model)	One dimensional – A type of model typically built using watercourse cross-section survey data to represent the watercourse and adjacent floodplain
2D (model)	Two dimensional – A type of hydraulic model typically built using LiDAR and site-specific topographic data to represent the wider floodplain
1D-2D (model)	A combination of 1D and 2D modelling (i.e. typically a representation of the watercourse and floodplain respectively)
AEP	Annual Exceedance Probability – the probability that a storm event will occur in any given year
Defra	Department for Environment, Food and Rural Affairs
DfI	Department for Infrastructure – one of nine departments in Northern Ireland responsible for regional strategic planning and development policy
EA	Environment Agency – non-departmental public body responsible for the protection and enhancement of the environment in England
ESTRY	Industry standard flood modelling software (1D engine built into TUFLOW)
FCA	Flood Consequence Assessment
FRA	Flood Risk Assessment
FFL	Finished Floor Level
Flood risk	The product of the frequency or likelihood of a flood event and the consequences (such as loss, damage, harm, distress and disruption)
FMP	Flood Modeller Pro – industry standard flood modelling software
HMR	Hydraulic Modelling Report
IDB	Internal Drainage Board
LA	Local authority
LiDAR	Light Detection and Ranging (i.e. Ground elevation data)
LLFA	Lead Local Flood Authority
Main river	A watercourse on which the relevant regulatory body (e.g. EA/NRW/SEPA/DfI) has permissive powers, but not a duty, to carry out maintenance, improvement, or construction work.
MIKE (11/21/FLOOD)	Industry standard flood modelling software
NGR	National Grid Reference
NRW	Natural Resources Wales – Welsh Government sponsored body responsible for managing the environment and natural resources of Wales
Ordinary watercourse	A river, stream, ditch, cut, sluice, dyke or non-public sewer that is not a designated main river, and for which the LA has flood risk management responsibilities and powers.

Term / Acronym	Definition
SEPA	Scottish Environment Protection Agency – non-departmental public body responsible for the protection and enhancement of the environment in Scotland
TN	Technical Note
TUFLOW	Industry standard flood modelling software

Short codes are used in the naming and referencing of model files, events and scenarios to ensure a consistent, high-quality naming convention is followed and to simplify communication of model results.

Table 2: Modelling Short Codes

Model Short Code	Definition
ABC	Model identifier
BL[x]-[y]	A y% blockage at location x (e.g. BL1-67 – 67% blockage at location “1”)
BR[x]-[y]	A y metre wide breach scenario at location x (e.g. BR2-50 – 50m wide breach at location “2”)
CC	Climate change
DEV	Post-development site layout
EXG	Existing (pre-development) site layout
MIT	Flood mitigation option
NC	“Normal conditions” A model setup representative of present channel and floodplain conditions throughout the study area – no additional structure blockages, defence breaches or pump failures
OPT	Development option
Q[x]	1 / x % AEP fluvial event
R[x]	1 / x % AEP pluvial event
ST[x]	Sensitivity test x (e.g. ST1 – channel and floodplain roughness +20%)
T[x]	1 / x % AEP tidal event

Introduction

Waterco has been commissioned to undertake a hydraulic modelling study in support of a Flood Consequences Assessment (FCA) for the proposed development of a Lidl store along Commercial Street, Risca, Newport, NP11 6EE, herein referred to as the site.

The main objective of this hydraulic modelling study is to quantify existing flood risk to the site and the change in flood risk elsewhere (if any) as a result of the proposed development. To enable this, the NRW linked 1D/2D¹ hydraulic model (EBBWLowerICM_5_V1.0_2018) has been utilised. This report summarises the hydraulic modelling works completed and should be read in conjunction with the associated FCA produced by Waterco.

Site Description

An overview of the site is detailed in Table 3. A location plan and an aerial photograph of site is included in Appendix A.

Table 3: Site Overview

Site Overview	
Location	The site comprises of a hardstanding area with industrial containers at Commercial Street, Pontymister, Risca, Newport, NP11 6EE.
NGR (Located at centre of site)	324398, 189871
Topographic Survey	A topographical survey of the site was undertaken by EDI Surveys Ltd in December 2023 and is included in Appendix B.
Watercourses	Ebbw River
Current Site Use	Industrial
Regulatory Body	NRW

Proposed Development

The proposed development comprises of a Lidl store and car park with associated landscaping. A proposed development plan is included in Appendix C. As part of the proposed development layout, the finished floor levels (FFLs) of the store will be set to 44.409m AOD as outlined by the client.

¹ A 1D/2D hydrodynamic model is comprised of a 1-Dimensional (1D) river network model (based on surveyed river cross-sections) coupled with a 2-Dimensional (2D) Digital Terrain Model (DTM) of the potential floodplain (created from LiDAR).

Flood Risk & History

Table 4 provides a brief overview of the flood risk pertaining to the site.

Table 4: Site Flood Risk

Site Flood Risk	
Type of Flood Risk	Fluvial
Primary Source of Flood Risk	The potential of out of bank flows from the Ebbw River that flows adjacent to the site.
NRW Development Advice Map	Site within Zone C1 – a developed area of the floodplain served by significant infrastructure including flood defences. The flood map is included in Appendix D.
NRW Flood Map for Planning (Published May 2024)	Flood Zone 2 - an area considered to be at moderate risk of fluvial and/or tidal flooding with an annual probability of flooding from rivers of between 1% and 0.1% and from the sea of between 0.5% and 0.1%. Flood Zone 3 - an area considered to be at high risk of fluvial flooding with an annual probability of the flooding greater than 1%. The flood map is included in Appendix D.
Historical Flood Risk	NRW Historic Flood Map indicates that the western side of the site was flooded in December 1979, due to the channel capacity being exceeded (no raised defences). No other information is available on this flood event.

Hydraulic Modelling

The information provided in this section details the modelling works carried out as part of this scheme. The current NRW Ebbw River integrated 1D/2D FMP-TUFLOW hydraulic model (EBBWLowerICM_5_V1.0_2018), defended (DEF) scenario has been used as a base for this scheme. The hydraulic model includes the Ebbw River and its tributaries.

The model log² provided with the FMP/TUFLOW model files (Model Reference: 15679-30063-027) should be consulted if further details are required. An explanation of the file, event and scenario naming convention used is provided in the model log.

A proposed hydraulic modelling and hydrological assessment methodology was sent to NRW for comment prior to commencing the model build (01/03/2024). NRW requested that the 2017 hydrology assessment was updated. Following this a revised methodology was submitted (24/08/2024) which NRW confirmed acceptance of (18/09/2024). All correspondence with NRW regarding the modelling approach is included in Appendix D.

Baseline Model Updates

The current NRW model has been updated to quantify the existing flood risk in the study area. A plan of the 1D/2D model extent is included in Appendix E. A number of updates have been made to the current NRW

² Doc ref: 15679-30063-Model_Log.xlsm

model to produce a more appropriate and site-specific model; these updates are detailed below:

- The upstream extent of the NRW Ebbw River model was truncated to EBBW_12077.3, downstream of the confluence of the Ebbw River and Sirhowy River. This is suitable distance upstream of the site (2.68km).
- The 2D domain, 1D Flood Modeller network, 1D-2D links and other model build shapefiles were clipped to the truncated upstream boundary. Existing NRW model shapefiles not within the updated model extent were removed.
- As requested by NRW, the hydrology assessment was updated in line with the current NRW guidance. The most upstream model inflow (EBBW_US) has been updated and applied to the most upstream cross section (EBBW_12077.3). Downstream of the site, the existing NRW model inflows remain as per the current NRW model (AE10, AE11 and AE12). For details on the hydrology assessment undertaken, see the FEC Record in Appendix F.
- Between EBBW_11528 and EBBW_7932 a number of 1D bank levels were updated to match the 2D defence levels where required.
- Bank levels have been enforced along the 1D-2D links at EBBW_10204 to EBBW_9857.5 and EBBW_9075 to EBBW_9019 using thick z-line features. The elevations within these z-lines are based on the 1D cross section bank levels.
- Existing site topographical survey levels have been applied in the form of a Triangular Irregular Network (TIN). The topographical survey is included in Appendix B. Areas not covered by the survey data have been represented using the LiDAR in the current NRW model which was found to better match the surveyed levels than the latest Welsh Government 1m LiDAR dataset (2020-2022).
- A blockage scenario at the B4591 Road Bridge has been considered. Details of the blockage scenario are detailed in Table 6. To simulate a blockage, a 'dummy' blockage unit was added to the 1D Flood Modeller network at EBBW_9284. Within this unit, the blockage proportion and the inlet and outlet loss coefficients were set to 0. During the blockage scenario simulations, a separate IED file was applied which contained the blockage proportion which was applied for the entire duration of a simulation.
- 1D cross section conveyance was reviewed and updated where required with the addition of panel markers.
- The simulation run time was extended to 25 hours to account for the updated hydrology.
- The model has been simulated in the latest software versions. Flood Modeller (version 7.1) and TUFLOW (version 2023-03-AF-iSP HPC).

Baseline Model Stability Fixes

The following updates were carried out to aid model stability.

- FLC coefficients have been applied to the 1D-2D links along the left bank, between nodes EBBW_9151.59 and EBBW_8883 and on both banks between EBBW_8149 and EBBW_7932. An FLC value of 0.5 was applied to represent the energy loss as water flows over the banks.

- The Dflood value was updated to 10 (4 in NRW model).
- The maximum iterations value was updated to 11 (35 in NRW model).
- Synchronise HPC timestep was unticked.
- A 2D roughness patch of 0.1 was added to the right bank at EBBW_7080.
- A 2D roughness patch of 0.05 was added to the left bank at EBBW_4008.

No other updates have been made.

Proposed Development Model Setup

To quantify the impact the proposed development has on flood extents and depths elsewhere, the proposed development has been incorporated into the baseline model. A plan of the proposed development can be found in Appendix C.

The proposed development modelled briefly comprises:

- Z-shape polygons have been used to represent the proposed FFL (44.409m AOD) and external levels as per the proposed levels strategy (included in Appendix C).
- Where proposed levels have not been specified, existing levels from the topographical survey have been used.
- A 2D roughness layer has been applied within the site boundary, specifying altered Manning's n values that correspond to the proposed development layout.

Model Simulation Setup

Primary Simulations

Primary simulations have been modelled for a range of fluvial design events and site scenarios during the defended (DEF) model scenario.

Table 5: Primary simulations under normal conditions (NC)

Fluvial Design Event (AEP)	Climate Change Allowance	Scenario	Site Layout
1%	-	Defended (DEF)	Existing (EXG)
1% CC1	+25%		
1% CC2	+70%		
0.1%	-		
0.1% CC1	+25%		
0.1% CC2	+70%		
1%	-		Proposed Development (DEV)
1% CC1	+25%		
1% CC2	+70%		
0.1%	-		
0.1% CC1	+25%		
0.1% CC2	+70%		

Blockage Simulations

Structure blockages have been assessed at one location with respect to the 1% AEP + 25% CC, 1% AEP + 70% CC and 0.1% AEP fluvial events, for both EXG and DEV site layout scenarios. Details of the studied blockages can be found in Table 6.

Blockage percentages were chosen based on a review of upstream land-use (e.g. woodland/vegetation), trash screen presence, channel slope and width, orifice shape and size and worst-case blockage estimate should the structure collapse.

Table 6: Structure blockage details

Blockage Location (Model Short Code)	Coordinates	Structure Details	Considerations	Blockage %	Fluvial Event (AEP)	Site Layout
BL1 (BL1-25)	324441, 189785	Road Bridge	Structure located immediately downstream of the site.	25	1% + 25% CC 1% + 70% CC 0.1%	EXG DEV

Model Limitations

Model Limitations

This section presents the steps taken throughout the modelling process to reduce uncertainty and improve confidence in the model outputs. Efforts have been made to assess and reduce the levels of uncertainty at each stage of the modelling process. The most appropriate available information has been used to construct the model to represent flooding mechanisms. The assumptions made are generally conservative for modelled water levels at the proposed scheme location and are therefore appropriate for the assessment of flood risk.

In the absence of available calibration data, sensitivity testing of two identified sources of uncertainty has been carried out and a review of the model's performance rating. Table 7 lists and ranks identified sources of uncertainty and the assumptions made as part of the model build process.

Limitations and Assumptions

Table 7: Sources of uncertainty and assumptions

Rank	Source	Selected method	Assumptions
1	Friction parameters	Manning's n roughness coefficient based on available guidance (Chow 1959)	Channel and floodplain roughness utilised in the current NRW model have been reviewed against available survey data and aerial photographs and found to be appropriate for use in this study.
2	Inflow hydrology	FEH Statistical	See FEC Record in Appendix F for details. In addition, sensitivity tests have been carried out to quantify the impact of a significant change in flows to the flood extents.
3	Topography	DTM and survey data	As per the current NRW model, 1m/2m resolution LiDAR data has been used to represent the floodplain topography and the watercourse alignment and bank levels are informed by surveyed cross sections and points and LiDAR data in the existing model. There has been no additional watercourse/bank level survey applied. The existing site layout have been informed by a topographical survey. For the proposed scheme, the existing ground levels were modified within the proposed scheme footprint using elevation polygons such as z shapes.
4	Grid size	4m cell size	This is suitable to represent the floodplain features across the model extents to an appropriate level of detail and to attain suitable model run times. Finer features have been incorporated into the grid using breaklines.
5	Structures	Flood Modeller Pro defaults.	There have been no changes to the hydraulic

Rank	Source	Selected method	Assumptions
			coefficients of structures applied within the current NRW model.
6	Change in climate condition	Predictions of anticipated change to watercourse flow	Climate change events selected based on TAN15 guidance.
7	Watercourse survey data	NRW EBBWLowerICM_5_V1.0_2018 model	The majority of the survey within the updated model extent is from the 2005 Ebbw FRM Survey. Check survey was obtained in 2016 which confirmed the existing survey was suitable. Although the survey is dated, it would be unproportionate to the objectives of this study for new survey data to be obtained.

Sensitivity Tests

Sensitivity testing of two sources of uncertainty (see Table 7) has been carried out. A total of four sensitivity tests have been carried out (ST1-4) with respect to the 1% AEP + 25% CC fluvial event. Details of these sensitivity tests can be found in Table 8.

Table 8: Sensitivity test details

Sensitivity Test Short Code	Varied Parameter	Adjustment	Fluvial Event (AEP)	Site Layout
ST1	Inflow	+20%	1% + 25% CC	EXG
ST2	Inflow	-20%	1% + 25% CC	EXG
ST3	Control Number Factor	+20%	1% + 25% CC	EXG
ST4	Control Number Factor	-20%	1% + 25% CC	EXG

The results of sensitivity tests ST1 and ST2 show that significantly varying the inflows used at the most upstream nodes by +/-20% respectively causes an increase/decrease in maximum water levels and flood extents throughout the model. During ST1, the increase can be attributed to the additional overtopping of the defences due to the increase in in-channel water levels. Additionally, the increase in flow causes the EBBW_9284 bridge to become surcharged causing an increase in upstream water levels adjacent to the site. Two differing flow estimation methods have been used to produce the final design hydrographs used in the model. Both flow estimation methods have been subject to scrutiny in accordance with EA guidance and detailed in the FEC Record (Appendix F)- the final design hydrographs are deemed reliable and robust.

The results of sensitivity tests ST3 and ST4 show that varying the Control Number Factor by +/-20% respectively does not affect the assessment of flood risk at the site.

Model Validation

The NRW historical flood outline map (included in Appendix D) states that the 27th December 1979 flood event that flooded the western extent of the site occurred due to the channel capacity being exceeded when there were no raised defences. Flood defences were constructed following this flood event. The Ebbw River

model utilised in this study contains NRW defences in the vicinity of the site, therefore a comparison would not be appropriate. Therefore, increased significance has been placed on sensitivity testing to understand confidence in the model outputs.

Model Performance and Stability

Run performance has been monitored throughout the model build process and then during each simulation carried out to ensure a suitable model convergence was achieved. As discussed above, a number of stability fixes have been carried out on the existing model in order to stabilise the model.

1D Stability

A review of the 1D log files shows that there are no comments, warnings or errors warranting attention.

The mass balance is within the tolerable limits of +/- 1% during all considered events.

There are instances of non-convergent timesteps reported (up to four), however these do not occur at cross sections near the site (EBBW_21160u, EBBW_4008, EBBW_7066, EBBW_7080 and EBBW_-2155) nor near time of peak flow and result in 0% of the simulations being unconverged. They are therefore considered acceptable and will not affect the assessment of flood risk at the site.

2D Stability

A review of the 2D log files shows that there are no comments, warnings or errors warranting attention.

The mass balance is within the tolerable limits of +/- 1% during all considered events.

During the 1% AEP + 70% CC event and higher, there is some minor glass walling in the 2D domain near The Parc Golf Club (NGR 327238, 183416) at the downstream boundary. This is over 10km from the site and will not affect the assessment of flood risk at the site, therefore is deemed acceptable.

Model Results

This section of the report discusses the results obtained from the hydraulic model simulations for the existing and proposed development scenarios, as well as the blockage scenario.

Maximum flood depth, velocity and hazard mapping has been provided for each primary simulation in Appendix G. Flood hazard ratings have been calculated in accordance with DEFRA document 'FD2320: Flood Risks to People' and EA guidance document 'Supplementary Note on Flood Hazard Ratings and Thresholds'.

Existing Site Layout Simulations (EXG)

The results of the hydraulic modelling show that the site is flood free up to and including the 1% AEP event. Almost the entire site remains flood free during the 1% + 25% CC event, with a very small area of the site in the west experiencing minor shallow flooding.

During the 1% AEP + 70% CC event and the 0.1% AEP event, floodwater overtops the left bank of the Ebbw River upstream of the site (between EBBW_9644.03 and EBBW_9580.73) and flows east towards the site. Floodwater overtopping the banks upstream of the site (up to the upstream model extent, node EBBW_12077.3 during the 0.1% AEP event) flows downstream across the Ebbw River floodplain and along Commercial Street until it reaches the western site boundary where it flows southeast across the site. Some

floodwater attempts to enter the Ebbw River adjacent to the site (EBBW_9397.91 to EBBW_9284), while some continues through the site, leaving along the northern and eastern boundary and flowing onto the B4591 and Commercial Street. Floodwater then flows east away from the site, flooding areas including Mill Street, Fields Road, Ty-Isaf Crescent, Park Road, Park Avenue and Springfield Road.

Post-Development Site Layout Simulations (DEV)

When the proposed development is considered, hydraulic modelling shows the site remains flood free up to and including the 1% AEP event, with only a small area of shallow flooding along the western boundary shown during the 1% + 25% CC event.

The proposed store is flood free up to and including the 1% AEP + 25% CC event.

Similar to the existing scenario, during the 1% AEP + 70% CC event and higher, floodwater overtops the left bank of the Ebbw River upstream of the site and flows east towards the site and enters the site along the western boundary. Some of the floodwater flows across the site where some is diverted around the proposed store and builds up behind it until the flood depths exceed the proposed finished floor level when it then flows south across the proposed store and car park. Some floodwater attempts to re-enter the channel along the southern boundary, whilst some leaves the site along the eastern boundary, overtopping the B4591 and Commercial Street, flooding areas downstream including Mill Street, Fields Road, Ty-Isaf Crescent, Park Road, Park Avenue and Springfield Road.

Blockage Scenario

During all considered events, a 25% blockage of the B4591 bridge causes the site and proposed store to flood during the existing and proposed development scenario.

During the 1% AEP + 25% CC event and higher existing scenario, the blockage of the bridge causes additional left bank overtopping onto the site. Overtopping between EBBW_9460.13 and EBBW_9397.91 flows northwest away from the site. Floodwater that has overtopped the banks downstream of EBBW_9397.91 to EBBW_9284 flows north across the site and onto Commercial Street and then northeast onto the B4591.

During the 1% AEP + 25% CC event and higher proposed development scenario, the blockage of the bridge causes additional left bank overtopping onto the site, from the southwestern corner where floodwater flows northwest out of the site and north along the access road. The floodwater then flows east along Commercial Street and behind the proposed store. There is also overtopping in the southeast corner of the site adjacent to the B4591 bridge where floodwater flows northeast, leaving the site and flowing onto the B4591. There is some overtopping of the left bank between EBBW_9397.91 and EBBW_9336.71. Floodwater is diverted around the proposed store until the depths exceed the proposed finished floor level. The floodwater then flows across the proposed store from the north and south and leaves the site via the eastern and northeast boundary.

Flood Risk Elsewhere

The potential impact of the proposed development on flood risk elsewhere has been quantified by comparing the equivalent EXG and DEV simulation results. To provide a detailed assessment of the relative changes in flood depths throughout the floodplain, a series of maximum flood depth difference maps have been created and are included in Appendix G.

The flood depth difference maps show that there is no change to flood risk elsewhere during and up to the 1% AEP + 25% CC event.

The flood depth difference maps show that during the 1% AEP + 70% CC event and 0.1% AEP event the proposed development is shown to cause a betterment (up to 300mm) and a minor (generally less than 30mm) increase in flood risk off site.

During the 1% AEP + 70% CC and 0.1% AEP events, there is an increase in flood depths west and north-west of the site along Commercial Street. During the 0.1% AEP event, there is a general increase of up to 30mm to land west and north-west of the site.

The modelled assessment of off-site impact has been made on the basis that the existing site is undeveloped. However, the site is currently used for storage of building supplies and materials which could remove flood storage from the floodplain. Full details of the impact of the development on flood risk elsewhere is provided in the FCA (reference 15679-FCA & Drainage Strategy-02).

Conclusions

Waterco has been commissioned to undertake a hydraulic modelling study in support of a Flood Consequences Assessment (FCA) for the proposed development of a Lidl store along Commercial Street, Risca, Newport, NP11 6EE, herein referred to as the site.

The main objective of this hydraulic modelling study is to quantify existing flood risk to the site and the change in flood risk elsewhere (if any) as a result of the proposed development. To enable this, the NRW linked 1D/2D hydraulic model (EBBWLowerICM_5_V1.0_2018) has been utilised. This report summarises the hydraulic modelling works completed and should be read in conjunction with the associated FCA.

The proposed development comprises of a Lidl store and car park with associated landscaping. As part of the proposed development layout, the finished floor levels (FFLs) of the store will be set to 44.409m AOD as outlined by the client.

The hydraulic modelling indicates that during the EXG and DEV normal condition simulations, the site is almost entirely flood free up to and including the 1% AEP + 25% CC event and the proposed store is flood free up to and including the 1% AEP + 25% CC event. A small area of the western edge of the site experiences shallow flooding during the 1% AEP + 25% CC event.

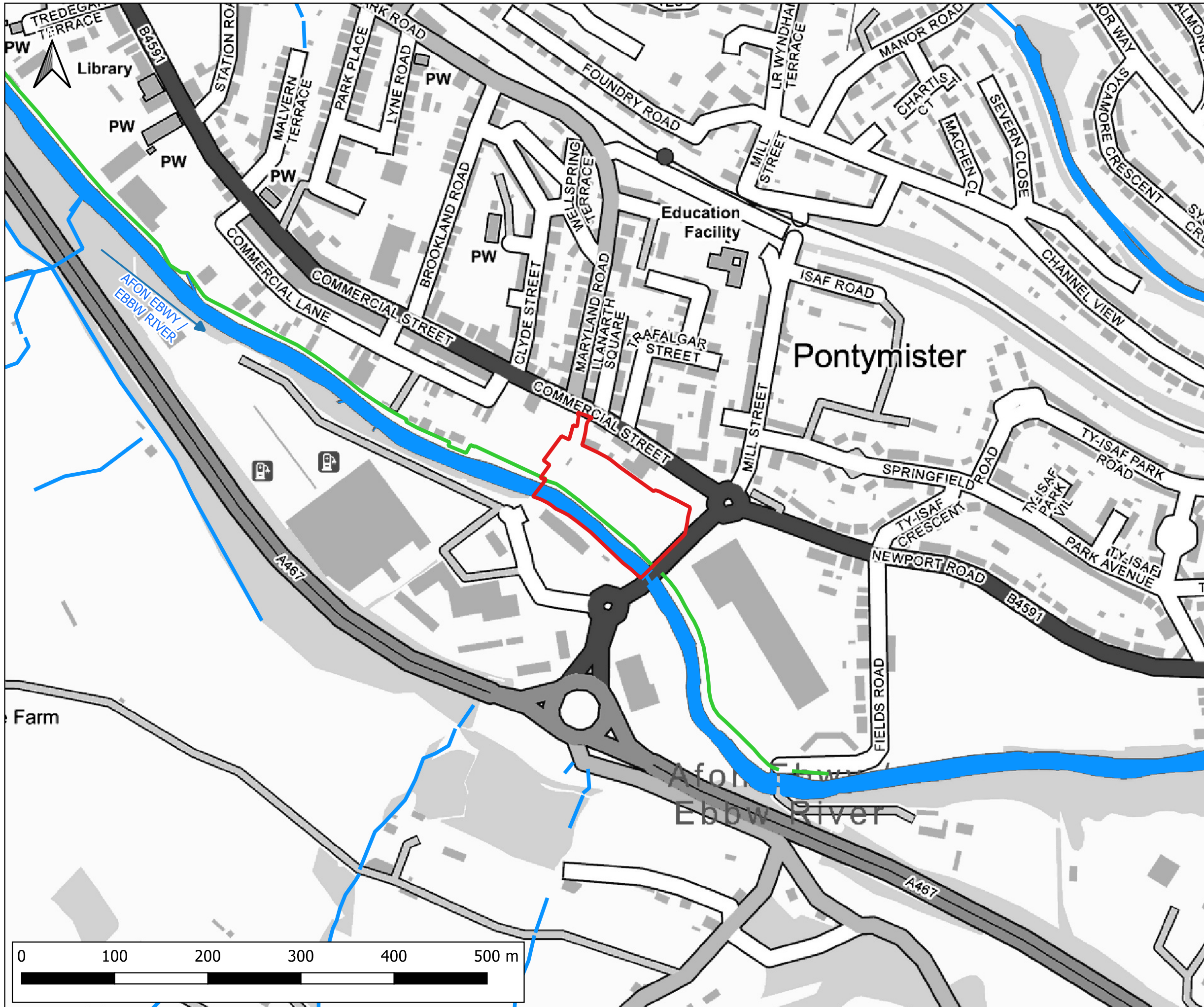
A 25% blockage of the B4591 bridge causes the site and proposed store to flood during the existing and proposed development scenario.

Recommendations

This hydraulic model is considered suitable to support the FCA being prepared by Waterco for the proposed development at Commercial Street, Risca.





This hydraulic model and associated report should be submitted to NRW for review and approval as a reasonable representation of the fluvial flood risk at the site from the Ebbw River when both the existing and proposed development levels / arrangements are considered.

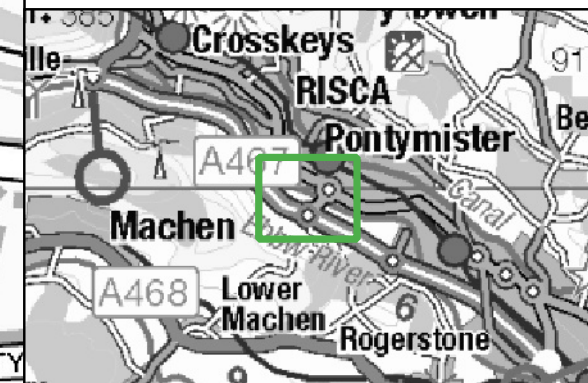
Appendix A Location Plan, Aerial Image and Photographs



Notes:
 1) All dimensions are in metres and all levels in metres above Ordnance Datum unless stated otherwise

LEGEND

-  Site Boundary
-  Location of NRW Flood Defences
-  Watercourses
-  Waterbodies



SCHEME:
 Land at Pontymister, Risca

PLOT TITLE:
 Location Plan

PLOT STATUS: FINAL DATE: 26-11-2024

DRAWN: MH	CHECKED: AA	APPROVED: LS	PLOT SCALE AT A3: 1:4000
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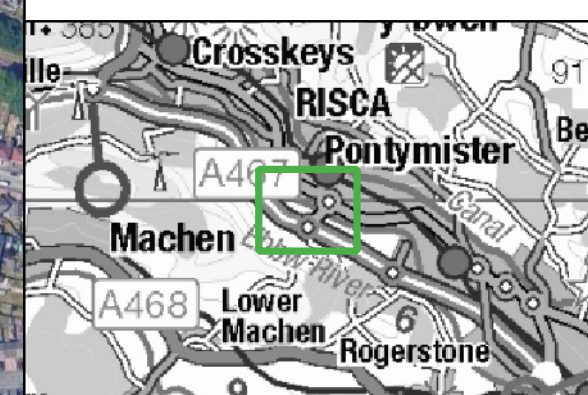
PLOT NAME: 15679_Location_Plan REVISION: -



Notes:
 1) All dimensions are in metres and all levels in metres above Ordnance Datum unless stated otherwise

LEGEND

 Site Boundary



SCHEME:
 Land at Pontymister, Risca

PLOT TITLE:
 Aerial Plan

PLOT STATUS: FINAL DATE: 26-11-2024

DRAWN: MH	CHECKED: AA	APPROVED: LS	PLOT SCALE AT A3: 1:4000
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PLOT NAME: 15679_Aerial_Plan	REVISION: -
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Appendix B Topographical Survey

Appendix C Proposed Development Plan

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DO NOT SCALE!
ALL DIMENSIONS SHOULD BE CHECKED ON SITE BEFORE WORK COMMENCES

DESIGN SUBJECT TO FOLLOWING:
HIGHWAYS ENGINEERING DESIGN AND COMMENT
TREE & TPO SURVEY
M&E DESIGN
FLOOD RISK ASSESSMENT
GROUND INVESTIGATION
LANDSCAPE ARCHITECTS DESIGN
LEVELS REVIEW
DRAINAGE DESIGN
TITLE DEED REVIEW

LIDL DESIGN BASED OFF OF LATEST BBS 2023 LIDL SPECIFICATION - FEBRUARY 2024

HIGHWAYS DRAWINGS BASED ON CORUN DRAWING: 23-00849 PL01 Rev 8 Potential Island Junction

SITE AREA = 10,710 SQ M / 2.64 ACRES

EXISTING TELECOMS POLE WITH OVERHEAD UTILITIES - SHOWN IN TURQUOISE

EXISTING TREES TO REMAIN - RPA TO BE SURVEYED - ANY EXCAVATION TO BE DONE AS PER SPECIALIST INSTRUCTION

SCHEDULE OF AREAS (TYPE 1300)

SALES	=1334 m ²
WAREHOUSE	=410 m ²
ANCILLARY	=218 m ²
GIA	=1962 m ²

EXISTING RIGHT OF WAY TO SLIGHTLY MODIFIED TO BE BROUGHT AWAY FROM THE PROPOSED ROADWAY AND STRAIGHTEN UP TO CONNECT TO BRIDGE CROSSING

SITE ACCESS AND HIGHWAYS BASED ON CORUN HIGHWAYS DRAWING: 23-00849 PL01 Rev B

PROPOSED LANDSCAPING BASED ON LANDSCAPE ARCHITECTS DESIGN. REFER TO CA 2024-RISCA-03 LANDSCAPEPROPOSALSOVERALL FOR MORE INFORMATION

Scale bar added, north point adjusted, general W 23/01/2025 annotation moved. KA
 Proposed landscaping scheme updated as per V 13/11/2024 Consolidation drawing 123 CA 2024 - RISCA-0 REV 8 KA
 U 11/11/2024 Proposed rear retaining wall location adjusted. KA
 Scheme updated as per new topographical survey received 04/11/24 and updated as per T 04/11/2024 Consolidation Landscaping scheme. KA
 S 04/10/2024 Lidl Flagpole added. KA
 Rev. Date Description Drawn

htcarchitects

York Place Studio
8 Britannia Street
Leeds
LS1 2DZ
T:(0113) 244 3457

W: www.htcarchitects.co.uk
E: info@htcarchitects.co.uk

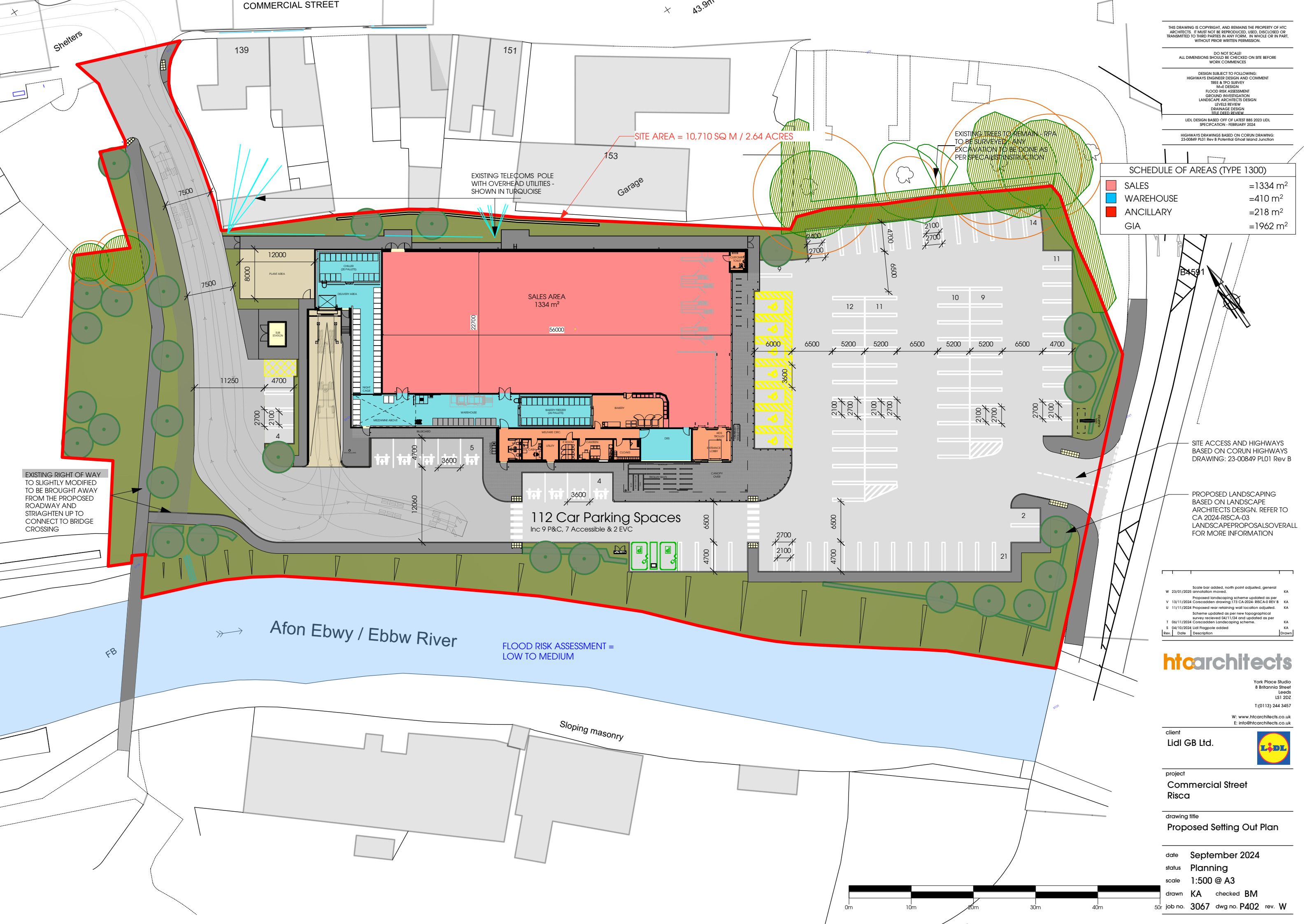
client
Lidl GB Ltd.



project
**Commercial Street
Risca**

drawing title
Proposed Setting Out Plan

date **September 2024**
status **Planning**
scale **1:500 @ A3**
drawn **KA checked BM**
job no. **3067** dwg no. **P402** rev. **W**



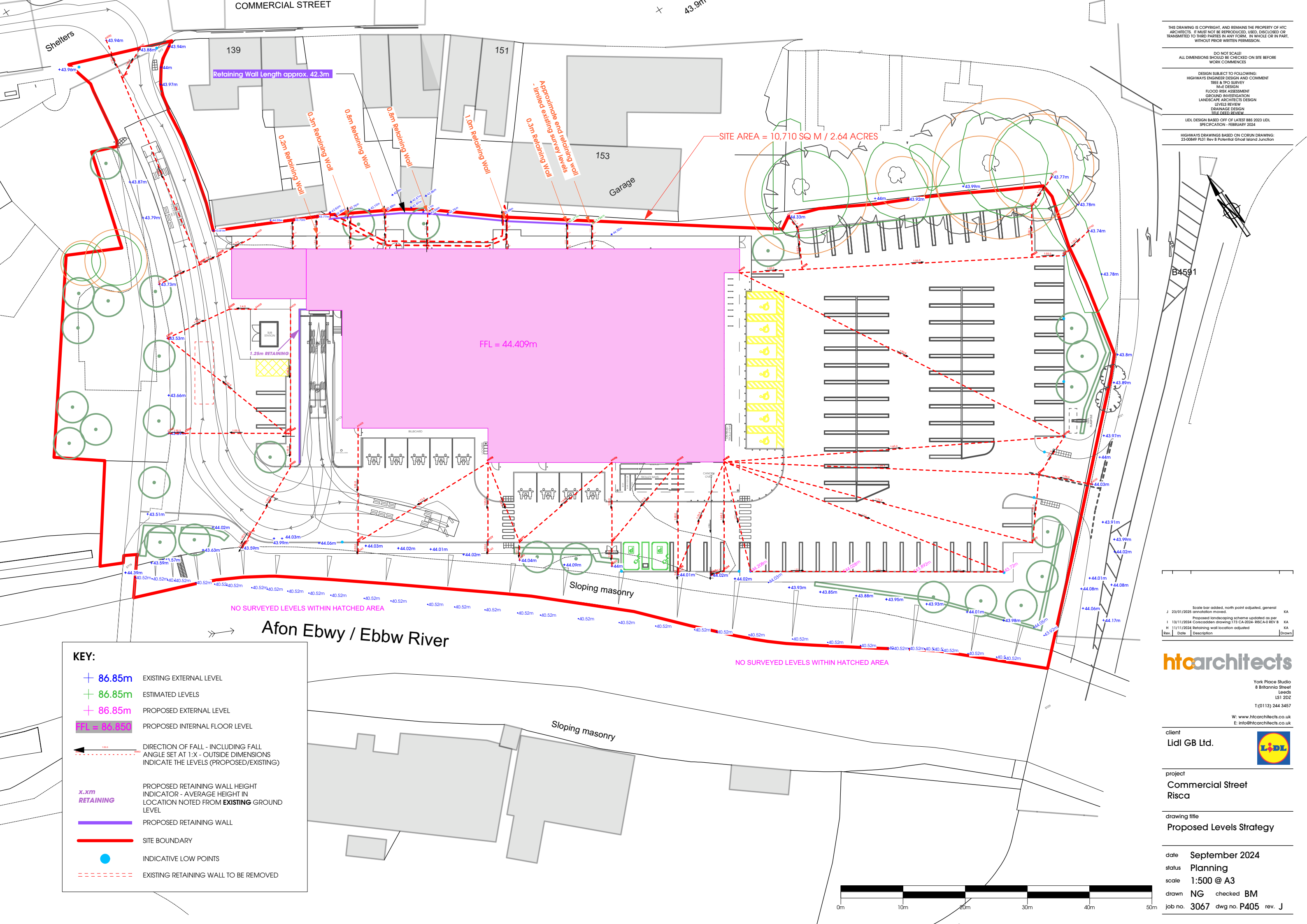
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DO NOT SCALE!
ALL DIMENSIONS SHOULD BE CHECKED ON SITE BEFORE WORK COMMENCES

DESIGN SUBJECT TO FOLLOWING:
HIGHWAYS ENGINEER DESIGN AND COMMENT
TRES & TPO SURVEY
M&E DESIGN
FLOOD RISK ASSESSMENT
GROUND INVESTIGATION
LANDSCAPE ARCHITECTS DESIGN
LEVELS REVIEW
DRAINAGE DESIGN
TITLE DEED REVIEW

LIDL DESIGN BASED OFF OF LATEST BBS 2023 LIDL SPECIFICATION - FEBRUARY 2024

HIGHWAYS DRAWINGS BASED ON CORUN DRAWING: 23-03849 PL01 Rev 8 Potential Ghost Island Junction



SITE AREA = 10,710 SQ M / 2.64 ACRES

Retaining Wall Length approx. 42.3m

FFL = 44.409m

NO SURVEYED LEVELS WITHIN HATCHED AREA

NO SURVEYED LEVELS WITHIN HATCHED AREA

KEY:

- + 86.85m EXISTING EXTERNAL LEVEL
- + 86.85m ESTIMATED LEVELS
- + 86.85m PROPOSED EXTERNAL LEVEL
- FFL = 86.850 PROPOSED INTERNAL FLOOR LEVEL
- DIRECTION OF FALL - INCLUDING FALL ANGLE SET AT 1:X - OUTSIDE DIMENSIONS INDICATE THE LEVELS (PROPOSED/EXISTING)
- x.xm RETAINING PROPOSED RETAINING WALL HEIGHT INDICATOR - AVERAGE HEIGHT IN LOCATION NOTED FROM EXISTING GROUND LEVEL
- PROPOSED RETAINING WALL
- SITE BOUNDARY
- INDICATIVE LOW POINTS
- - - EXISTING RETAINING WALL TO BE REMOVED


Scale bar added, north point adjusted, general KA
J 23/01/2025 annotation moved.

Rev.	Date	Description	Drawn
1	13/11/2024	Proposed landscaping scheme updated as per	KA
2	11/11/2024	Retaining wall location adjusted	KA

htcarchitects

York Place Studio
8 Britannia Street
Leeds
LS1 2DZ
T:(0113) 244 3457
W: www.htcarchitects.co.uk
E: info@htcarchitects.co.uk

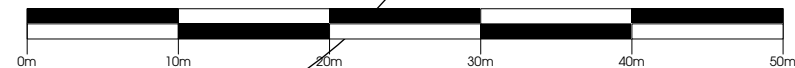
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Lidl GB Ltd.



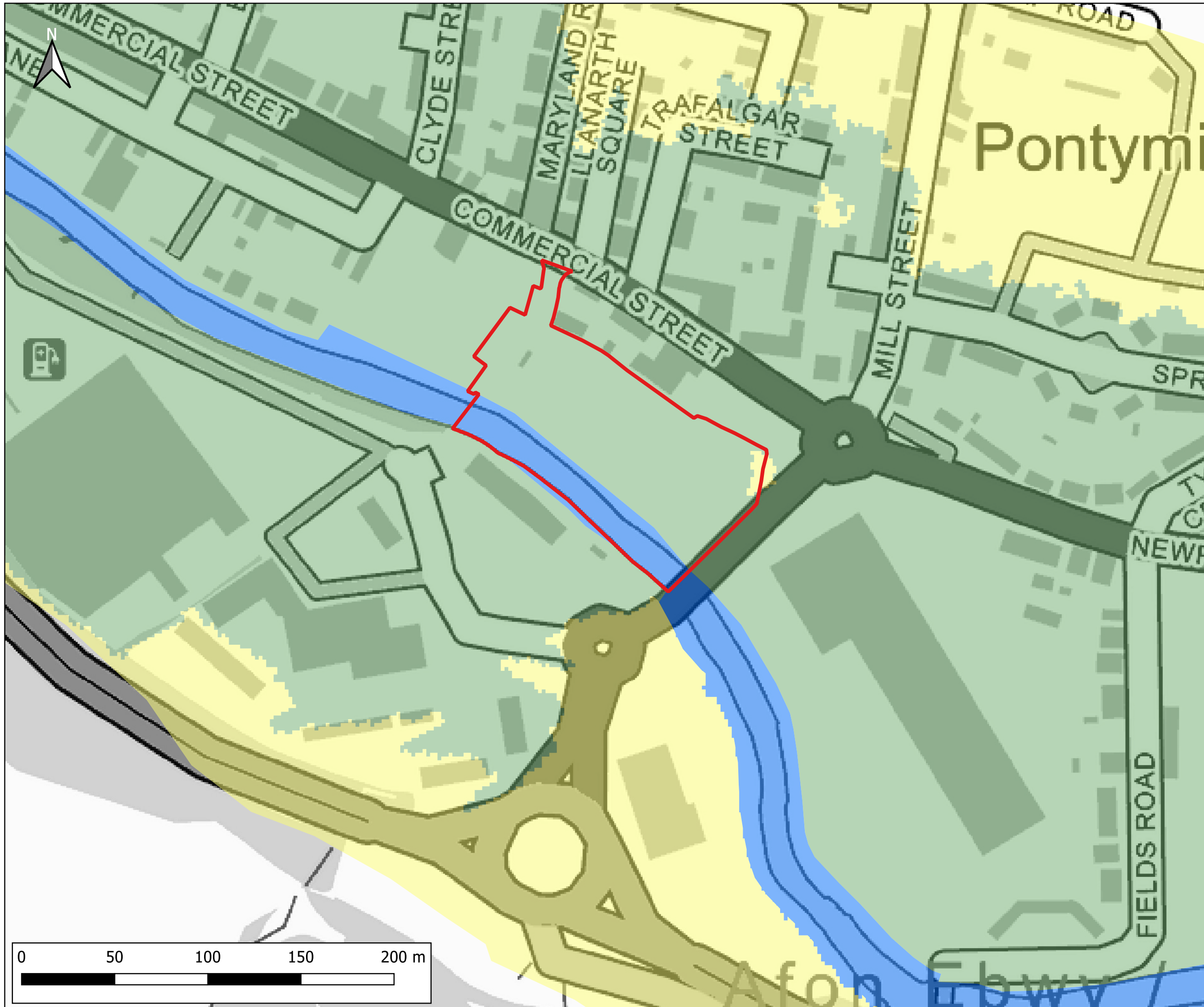
project
**Commercial Street
Risca**

drawing title
Proposed Levels Strategy

date **September 2024**
status **Planning**
scale **1:500 @ A3**
drawn **NG** checked **BM**
job no. **3067** dwg no. **P405** rev. **J**



Appendix D NRW Flood Maps and Correspondence



Notes:
 1) All dimensions are in metres and all levels in metres above Ordnance Datum unless stated otherwise

LEGEND

- Site Boundary
- Development Advice Map (TAN15)
 - Zone A
 - Zone B
 - Zone C1
 - Zone C2



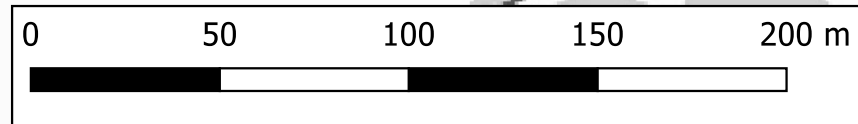
SCHEME:
 Land at Pontymister, Risca

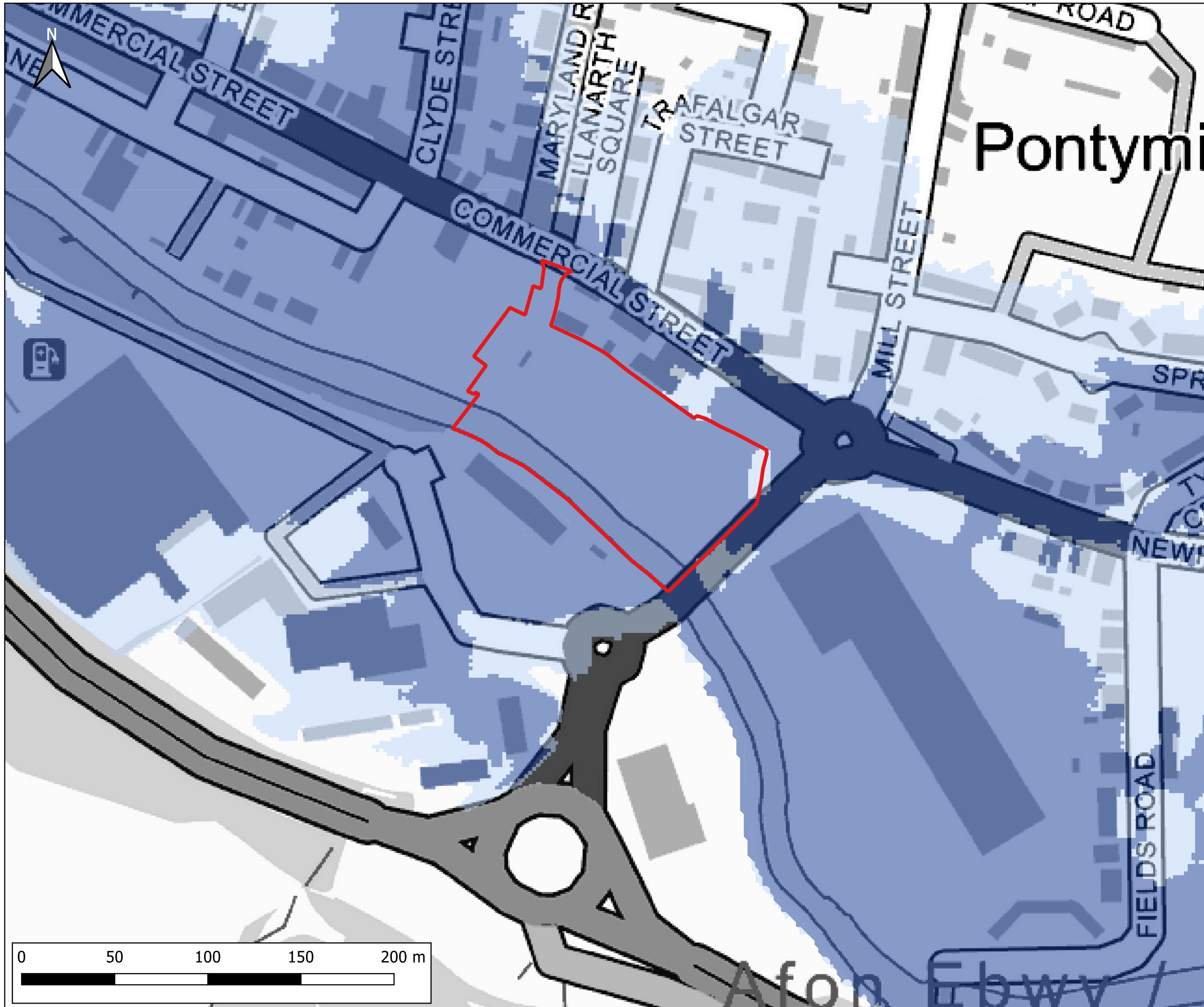
PLOT TITLE:
 NRW Development Advice Map
 Data published October 2022

PLOT STATUS: FINAL DATE: 26-11-2024

DRAWN: MH	CHECKED: AA	APPROVED: LS	PLOT SCALE AT A3: 1:2000
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

PLOT NAME: 15679_NRW_DAM_Map	REVISION: -
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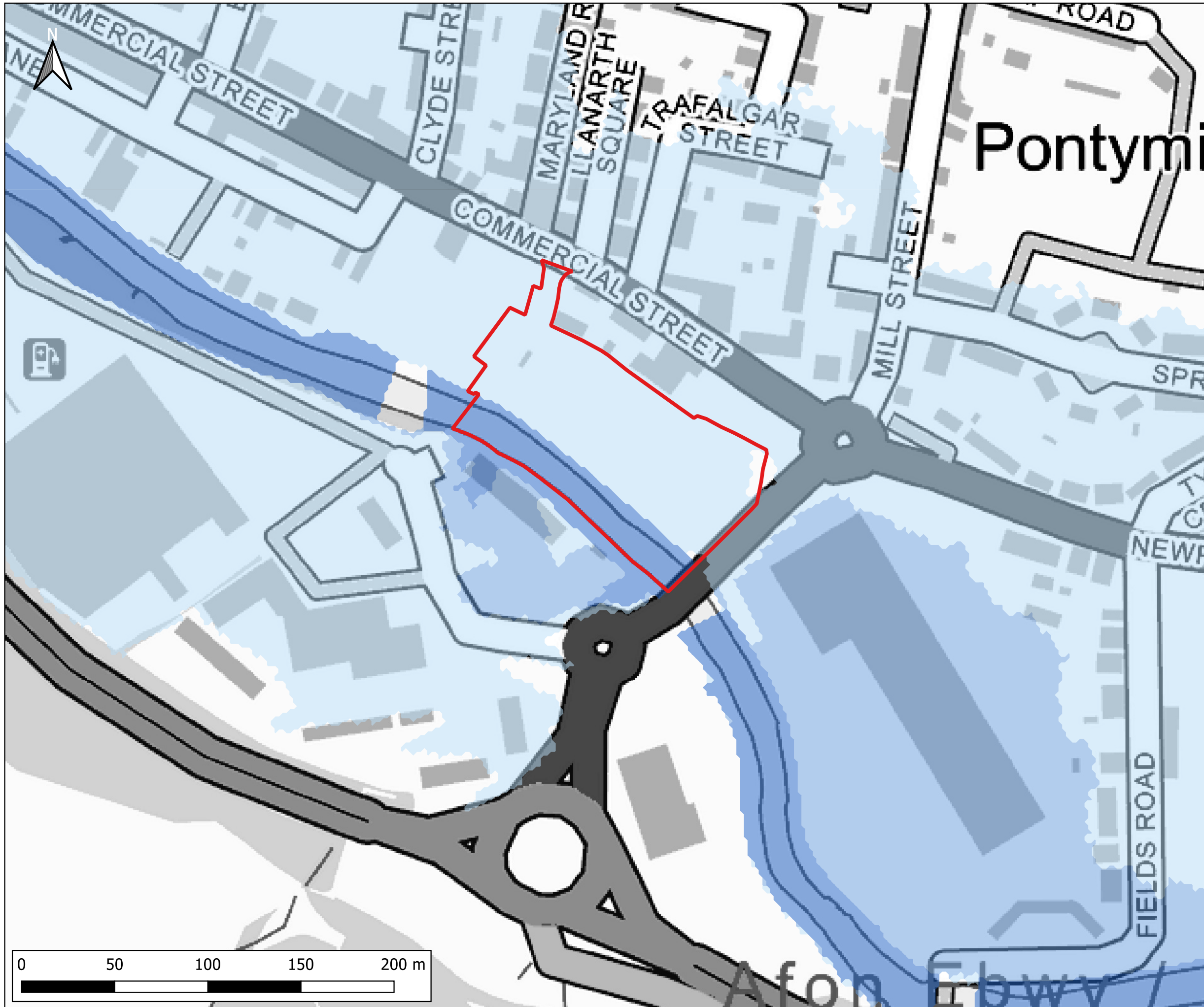
Notes:
 1) All dimensions are in metres and all levels in metres above Ordnance Datum unless stated otherwise
 2) Flood Zone 3 displays the extent of flooding from rivers with a 1% AEP or greater of flooding in a given year including and allowance for climate change and the sea with a 0.5% AEP or greater of flooding in a given year including and allowance for climate change
 3) Flood Zone 2 displays the extent of flooding from rivers with a less than 1% AEP but greater than or equal to 0.1% AEP of flooding in a given year including and allowance for climate change and the sea with a less than 0.5% AEP but greater than or equal to 0.1% AEP of flooding in a given year including and allowance for climate change

LEGEND

-  Site Boundary
- NRW Flood Map for Planning
 -  Flood Zone 2
 -  Flood Zone 3



CLIENT:			
			
 www.waterco.co.uk			
SCHEME:			
Land at Pontymister, Risca			
PLOT TITLE:			
NRW Flood Map for Planning (Rivers and Sea) Data published May 2024			
PLOT STATUS:			DATE:
FINAL			26-11-2024
DRAWN:	CHECKED:	APPROVED:	PLOT SCALE AT A3:
MH	AA	LS	1:2000
PLOT NAME:			REVISION:
15679_NRW_FMFP			-



Notes:
 1) All dimensions are in metres and all levels in metres above Ordnance Datum unless stated otherwise

LEGEND

- Site Boundary
- Flood Risk from Rivers**
 - High (greater than 3.3% AEP)
 - Medium (between 1% and 3.3% AEP)
 - Low (between 0.1% and 1% AEP)



SCHEME:
 Land at Pontymister, Risca

PLOT TITLE:
 NRW Flood Risk from Rivers
 Data published May 2024

PLOT STATUS: FINAL DATE: 26-11-2024

DRAWN: MH	CHECKED: AA	APPROVED: LS	PLOT SCALE AT A3: 1:2000
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PLOT NAME: 15679_NRW_Flood_Risk_from_Rivers	REVISION: -
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